## CHAPTER 4 FACILITY REQUIREMENTS

## 4.1 INTRODUCTION

To properly plan for the future requirements of Southwest Oregon Regional Airport, it is necessary to translate the forecasts of aviation demand into the specific types and quantities of facilities that are projected to be needed. The demand for new or expanded facilities is often driven by capacity shortfalls that leave an airport unable to accommodate forecast growth with existing facilities. However, the requirements for new or improved facilities can also be driven by other circumstances. For example, facilities may be needed to comply with updated standards developed and adopted by the FAA or other regulatory agencies, accommodate the strategic vision for the Airport, or replace outdated or inefficient facilities that are prohibitively costly to maintain or modernize. These circumstances can have a significant impact on future needs and have been considered in this analysis for the Airport.

The findings of the capacity analyses and facility requirement determinations form the foundation for the identification of development alternatives in subsequent chapters of this plan. Evaluation of those alternatives will allow Airport staff to create a development plan to meet future demand. Critical future investment decisions will be based on these analyses.

The facility requirements analysis begins with a review of emerging industry trends that may influence the need for future facilities. The remaining portion of this chapter is devoted to assessment of each of the following major issues and functional facility areas of Southwest Oregon Regional Airport:

- Emerging Trends
- Meteorological Conditions
- Airport Design Classifications
- Airfield Design
- Airspace, Navigational Aids, and Visual Aids Requirements
- Passenger Terminal Area
- Vehicle Access, Circulation, and Parking Requirements
- Aviation Support Facilities Requirements

This chapter concludes with a section that summarizes the key findings of the facility requirement assessments that will be carried forward to the identification and evaluation of alternatives.



## 4.2 EMERGING TRENDS

The aviation industry is changing rapidly and this evolution may have a significant impact on the size, quantity, and type of facilities needed to accommodate future demand. The rapid pace of change in the aviation industry is expected to continue. All airport master planning efforts should examine industry trends and identify those that will influence their capacity needs. Some of the emerging trends in the aviation industry that should be considered in the master plan for Southwest Oregon Regional Airport include:

- Airports may need to develop additional revenue streams beyond traditional aviation-related revenues from landing fees, terminal rentals, hangar leases, and fuel taxes. This can be accomplished by developing alternative uses for airport property that is surplus to current aviation-related functions.
- There is a growing need for airports to find ways for optimizing concessions programs/facilities in terminal buildings.
- The design and layout of terminal buildings is evolving as new technology and security requirements are altering the relationships between the airport, airlines, and passengers.
- Sustainability initiatives are pushing airports toward energy-efficient and environmentally responsive facilities. Advances in construction techniques and increasing awareness of operational impacts have resulted in the adoption of green building practices.
- A major revision to the FAA Advisory Circular (AC) 150/5300-13, Airport Design is expected to be published in September 2012. The Draft AC 150/5300-13A was released in May 2, 2012, and includes AC design standard changes that address airfield safety and airfield design. A significant portion of design guidance published in FAA Engineering Brief (EB) No. 75: Incorporation of Runway Incursion Prevention into Taxiway and Apron Design, will be incorporated as design standards into AC 150/5300-13A, likely rendering EB No. 75 obsolete after the AC revision is published. These changes may drive some changes to the geometry of key airfield features and land acquisition strategies.

In general, many of the emerging trends in the aviation industry focus on the implementation of technology, lowering operational cost for airport facilities while at the same time reducing environmental impacts. The other major influence will be an increasing need to expand airport revenue streams beyond the traditional aviation related activities.



## 4.3 METEOROLOGICAL CONDITIONS

Climate conditions have an influence on aircraft performance, as well as airfield dimensional and separation standards. Temperature, precipitation, winds, visibility, and typical cloud ceiling heights are important climate factors used to assess weather conditions and the aircraft operational impacts associated with use of Runways 04/22 and 13/31.

## 4.3.1 <u>Climate Summary</u>

The average annual temperature for North Bend is 52° Fahrenheit (F), ranging from 66°F in August and September to 39°F in January. On average the temperature exceeds 80°F six days annually, and exceeds 60°F (standard temperature) more than 152 days. The average annual precipitation is 61.9 inches. On average, visual flight rules (VFR) conditions (ceiling of at least 1,000 feet and visibility of at least three miles) are experienced over 86 percent of the time (316 days), while instrument flight rules (IFR) conditions (ceiling of less than 1,000 feet or visibility of less than three miles) occurred within the remaining 14 percent (49 days) of the year.

## 4.3.2 <u>Runway Orientation and Wind Analysis</u>

Runway wind coverage analysis was conducted using the FAA's Airports GIS Airport Design Tools Wind Analysis, with data supplied by National Climatic Data Center from the weather reporting station at Southwest Oregon Regional Airport during the period from 2000 through 2009. FAA planning standards recommend that the runway system provide a minimum of 95 percent wind coverage. If a single runway cannot provide this level of coverage, then a crosswind runway is warranted.

As shown in Figure 4-1, Runway 04/22 provides 82.50 percent or better wind coverage for allweather conditions with a 10.5 knot crosswind component, while Runway 13/31 provides 92.79 percent or better wind coverage. When combined, Runway 04/22 and Runway 13/31 provide 97.14 percent wind coverage during all-weather conditions with a 10.5 knot crosswind component. During inclement weather conditions, Runway 04/22 provides 88.15 percent or better instrument flight rules (IFR) wind coverage with a 10.5 knot crosswind component, while Runway 13/31 provides at least 94.15 percent.

The current runway configuration is adequate in respect to providing sufficient wind coverage; however, neither Runway 04/22 nor Runway 13/31 can independently provide the FAA recommended 95 percent all-weather or IFR wind coverage for the 10.5 knot crosswind component. Consequently, both runways are important to maintain at present design standards to ensure adequate wind coverage.



	Wind Coverage Provided Under All-Weather Conditions					
	10.5 Knots 13 Knots 16 Knots 20 Knots					
Runway	(% Component)	(% Component)	(% Component)	(% Component)		
Runway 04/22	82.50%	87.72%	93.53%	97.55%		
Runway 13/31	92.79%	96.40%	98.98%	99.76%		
Combined Runways	97.14%	98.99%	99.79%	99.98%		
	Wind Coverage Provided Under IFR Conditions					
	10.5 Knots	13 Knots	16 Knots	20 Knots		
Runway	(% Component)	(% Component)	(% Component)	(% Component)		
Runway 04/22	88.15%	93.15%	97.55%	99.31%		
Runway 13/31	94.15%	97.04%	99.00%	99.75%		
Combined Runways	98.02%	99.36%	99.86%	99.97%		

# Table 4-1 ALL WEATHER AND IFR WIND COVERAGE

Source: National Oceanic and Atmospheric Administration, Observation Period 2000-2009, Station 72691, North Bend, Oregon.

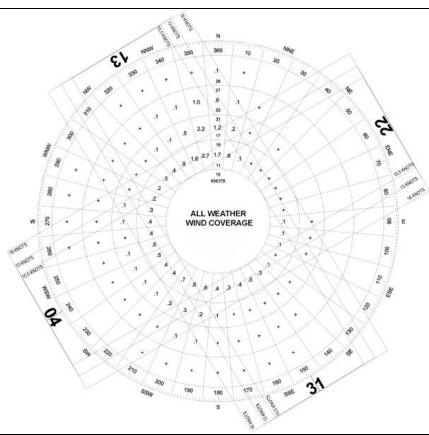


Figure 4-1
ALL WEATHER WIND ROSE

Sources: National Oceanic and Atmospheric Administration, Observation Period 2000-2009, Station 72691, North Bend, OR; FAA All Weather Wind Rose Form; and RS&H, 2012.



## 4.4 AIRPORT DESIGN CLASSIFICATION

Airport design classification identifies the FAA's Airport Reference Code (ARC) and critical aircraft for the Airport expected during this planning period (2010-2030). This section identifies the airport's national role and service level, the ARC, critical aircraft, and related airport design standards.

## 4.4.1 <u>Airport Role and Service Level</u>

Southwest Oregon Regional Airport is identified in the FAA's *National Plan of Integrated Airports System (NPIAS)* as a Primary/Non-Hub commercial service facility. A Primary/Non-Hub, as defined by the FAA, is a commercial service airport that accommodates more than 10,000 annual passenger enplanements, but less than 0.05 percent of total annual U.S. passenger enplanements. Currently, within the NPIAS there are 244 Non-Hub airports in the U.S., based upon the 2011-2015 NPIAS Report. According to the FAA Terminal Area Forecast, the Airport enplaned approximately 22,585 passengers in 2010, and is projected to remain a Non-Hub facility throughout the 20-year airport master planning horizon. The 2011-2015 NPIAS Report indicates that the largest Non-Hub airport enplaned approximately 367,874 passengers. This classification is used for FAA planning and funding purposes.

## 4.4.2 <u>Airport Design Classification and Airport Reference Code (ARC)</u>

The Airport Reference Code (ARC) is a system developed by the FAA to relate facility design criteria to the operational and physical characteristics of the airplane types that will operate at a particular airport. The ARC has two components relating to the airport design aircraft. The first component, depicted by a letter, is the Aircraft Approach Category (AAC) and relates to aircraft approach speed. The second component, depicted by a Roman numeral, is the Airplane Design Group (ADG) and relates to airplane wingspan and tail height. In the case of ADG I, an additional designation of "small aircraft only" relates to aircraft with maximum gross weights of 12,500 pounds or less. Generally, aircraft approach speed applies to runway length and related features. Airplane wingspan and tail height primarily relates to runway-taxiway separation and width criteria.

Airports expected to accommodate only small piston-engine airplanes normally fall into ARC A-I or B-I. Airports serving larger general aviation and commuter-type planes are usually ARC B-II or B-III. Small to medium-sized airports serving air carriers are usually ARC C-III, while larger air carrier airports are usually ARC D-IV or D-V. The elements that comprise the ARC are described below.

*Aircraft Approach Category* (ACC) - A grouping of aircraft based on 1.3 times the aircraft stall speed in landing configuration at the maximum certificated landing weight. The categories are as follows:

Category A: Approach speed less than 91 knots Category B: Approach speed 91 knots or more but less than 121 knots Category C: Approach speed 121 knots or more but less than 141 knots Category D: Approach speed 141 knots or more but less than 166 knots Category E: Approach speed 166 knots or more



*Airplane Design Group (ADG)* - A grouping of airplanes based on wingspan or tail height. When wingspan and tail height place an aircraft in two different groups, the more demanding group is used. The groups are as follows:

- Group I: Up to but not including 49 feet (15 m) wingspan or tail height up to but not including 20 feet
- Group II: 49 feet (15 m) up to but not including 79 feet (24 m) wingspan or tail height from 20 up to but not including 30 feet
- Group III: 79 feet (24 m) up to but not including 118 feet (36 m) wingspan or tail height from 30 up to but not including 45 feet
- Group IV: 118 feet (36 m) up to but not including 171 feet (52 m) wingspan or tail height from 45 up to but not including 60 feet
- Group V: 171 feet (52 m) up to but not including 214 feet (65 m) wingspan or tail height from 60 up to but not including 66 feet
- Group VI: 214 feet (65 m) up to but not including 262 feet (80 m) wingspan or tail height from 66 up to but not including 80 feet

The forecast of operations by Airport Reference Code for the years 2010 through 2030 is provided in Figure 4-2. Key conclusions that can be reached from this projection are:

- The majority of aircraft operations will remain in the A-I/B-I and A-II/B-II segments; however, the growth in the A-I/B-I segment is nominal. This mirrors the nationwide trend in the decreased utilization of small piston-powered aircraft.
- The least amount of operations will be conducted by Airplane Design Group III aircraft. The A-III/B-III segments are expected to decline, while the C-III/D-III segment is expected to slightly increase while maintaining their existing share of operations. Although this segment is declining, the majority of ADG III operations occur during the peak summer months.
- Operations within the A-II/B-II, C-I/D-I, and C-II/D-II segments will continue to grow at a steady rate and increase their share of total projected aircraft operations.



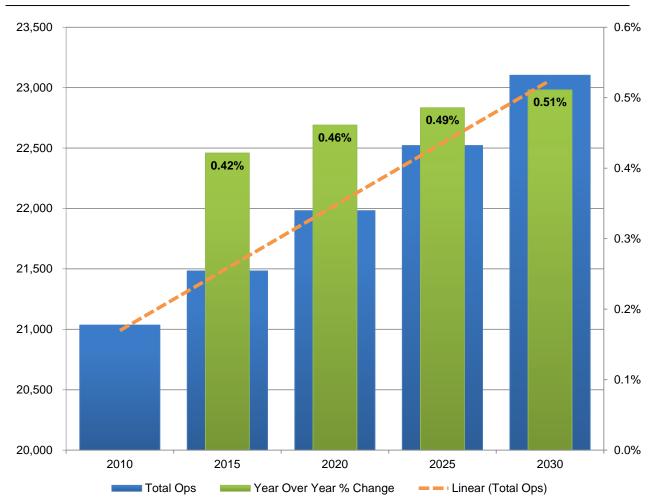


Figure 4-2 20-YEAR TOTAL AIRCRAFT OPERATIONS GROWTH

	A-I/B-I	A-II/B-II	A-III/B-III	C-I/D-I	C-II/D-II	C-III/D-III	C-IV/D-IV	Total
2010	8,835	6,521	210	2,104	3,155	210	0	21,035
2015	8,915	6,767	215	2,148	3,222	215	0	21,482
2020	9,013	6,924	220	2,198	3,407	220	0	21,982
2025	9,121	7,207	113	2,252	3,603	225	0	22,521
2030	9,357	7,393	116	2,310	3,696	231	0	23,103
					Compour	nd Annual G	Frowth Rate	0.5%

Source: Reynolds, Smith and Hills, Inc., 2012.

Note: Totals may differ slightly due to rounding.



To determine the design standards for a particular airport or airport facility, the ARC for the largest family of aircraft conducting (or expected to conduct) at least 500 annual operations (combination of takeoffs and landings) at the airport is identified as the critical aircraft. This critical aircraft, described by approach speed, wingspan, and tail height, serves as the basis for determining the Airport's design, structure, and equipment needs for airfield, runway, and terminal area facilities.

As discussed in **Chapter 3**, **Aviation Demand Forecast**, the previous master plan study determined that the "most demanding" type of aircraft at the Airport is the Bombardier Dash 8-200 (A-III). While commercial jet service was initiated during the summer months of 2011 with the 50-seat Bombardier CRJ-200 regional jet aircraft (C-II), the current scheduled commercial service aircraft is the Embraer E-120 Brasilia (B-II),. Corporate jets using the Airport typically fall within the ARC B-I through D-III categories. Airport landing records indicate corporate jet operations conducted by a variety of aircraft, such as the Falcon 50 (B-II), Bombardier Global Express (C-III), and the Gulfstream V (D-III). Corporate jet operations in the Airplane Design Group III category occur more frequently during the peak summer months, a trend that is expected to continue throughout the planning period. (See **Appendix E, Business Jet Data**, for a list of business jet aircraft characteristics, including Airport Reference Codes). The forecast chapter, as approved by the FAA, concludes that the CRJ-200 will be the critical aircraft for Southwest Oregon Regional Airport.

It is important to note that specific areas of an airport can be identified to accommodate various categories of aircraft. That is, each runway or aircraft movement area may be designated for a different type aircraft. For example, one runway may be designed to accommodate general aviation aircraft and another designed to serve commercial services aircraft.

Based on the expected operations by Airport Reference Code, Table 4-2 identifies the existing and future ARC designated for Runways 04/22 and 13/31 at Southwest Oregon Regional Airport. The existing ARC for both Runways 04/22 and 13/31 is B-III. Prior planning documents indicate the future ARC for Runway 04/22 is C-III, and in 2009 the parallel taxiway serving the primary runway (Taxiway C) was relocated to meet ARC C-III design standards. Based on the expected aircraft operational forecast and previous planning and design, the future ARC for Runway 04/22 will remain C-III, and the ARC for Runway 13/31 is expected to remain as B-III.



Runway	Aircraft Approach Category	Approach S	peed (knots)
	Category A	< 91	knots
Runways 04/22 & 13/31 (Existing) -	Category B	91 to < 1	21 knots
Runway 04/22 (Future) –	Category C	121 to < 141 knots	
	Category D	141 to <	166 knots
	Category E	> 166 knots	
Runway	Airplane Design Group	Wingspan (ft)	Tail Height (ft)
	Group I	< 49'	< 20
	Group II	49' to < 79'	20' to < 30'
Runways 04/22 & 13/31 —	→ Group III	70' to < 118'	30' to < 45'
	Group IV	118' to < 171'	45' to < 60'
	Group V	171' to < 214'	60' to < 66'
	Group VI	214' to < 262'	66' to < 80'

# Table 4-2 AIRPORT REFERENCE CODE – EXISTING AND FUTURE

Sources: FAA Advisory Circular 150/5300-13 and Reynolds, Smith, and Hills Inc., 2012.

Note: Combined, the 'approach category' and 'design group' yields the Airport Reference Code (ARC) which determines the type of airplane (family) that the airport is designed to accommodate.

In conclusion, although Runway 04/22 and Runway 13/31 are designed to different future ARC standards, the single ARC designation for Southwest Oregon Regional Airport will be C-III. Figure 4-3 shown below illustrates the representative aircraft for each Airplane Design Group (ADG). ADG aircraft types and representative aircraft shown in bold denote aircraft that frequently operate at Southwest Oregon Regional Airport.



FAA Airplane Design Group	Aircraft Type	Representative Aircraft	
	Single/Twin-Engine Piston	Cessna 150/210, Baron 55/58	
I	Turboprops	Beechcraft King Air B100, Rockwell Turbo Commander	PH-WBR
	Small Cabin Business Jets	BeechJet 400A, Learjet 25	
	Turboprops	Beechcraft Super King Air, Embraer EMB-120	
" →	Business Jets	Cessna Citation, Gulfstream II/III/IV	
	Smaller Airline Regional Jets	Embraer ERJ-135, Bombardier CRJ-200	-16
	Large Regional Jets	Embraer ERJ-170/195	~ ~
Ⅲ →	Short-Haul Narrowbody Transports	DHC Dash 7/8, Boeing 737, Airbus A319/20	Contraction of Contraction
	Large Corporate Jets	Bombardier Global Express, Gulfstream V	
IV	Large Long-Haul Narrowbody Transports	Boeing 757	***
	Widebody Transports	Airbus A310, Boeing 767	
V	Large Widebody Transports	Boeing 777, Boeing 747, Airbus A340	
VI	Large Heavy Lifting Transports	Boeing 747-8F, Airbus A380, Lockheed C-5 Galaxy	

Figure 4-3
AIRPLANE DESIGN GROUP TYPE AND REPRESENTATIVE AIRCRAFT

Sources: FAA Advisory Circular 150/5300-13 and www.airliners.net.

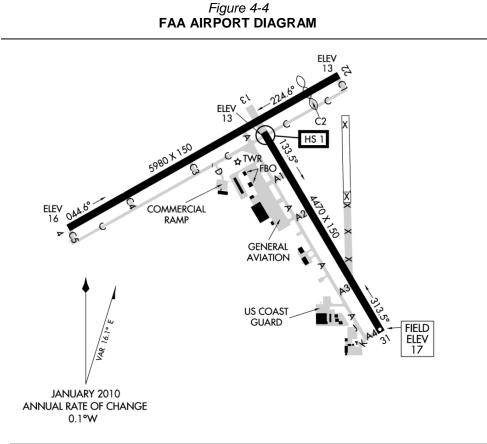


## 4.5 AIRFIELD DESIGN

This section describes the airfield facility needs, as well as the methods and planned timing upon which the facility requirements have been determined. Areas examined include:

- Airfield Capacity
- Runway Design
- Taxiway Design
- Pavement Strength
- Modifications to Standards

Figure 4-4 provides an illustration of the airfield geometric design and site layout, as published by the FAA. The airfield geometric design and site layout are determined by application of airport design standards contained in FAA Advisory Circular 150/5300-13, *Airport Design*.



Source: FAA Airport Diagram, North Bend/Southwest Oregon Regional (OTH), NW-1, 2012.



## 4.5.1 <u>Airfield Capacity</u>

Airfield capacity is an estimate of the number of aircraft that can be processed through the airfield system during a specific period without unacceptable delays. The airfield capacity analysis identifies the annual capacity of the airfield, referred to as the annual service volume (ASV), and the hourly capacity, based on the current operational characteristics.

Major factors that affect airfield capacity include the runway configuration, air traffic control procedures, weather conditions, and aircraft fleet mix. For instance, required separations between aircraft are greatly increased during inclement weather. As a result, the number of aircraft that can operate at an airport under instrument meteorological conditions will be much less than during visual meteorological conditions.

The goal of the analysis is to determine the airfield capacity and the ability of the runways to handle peak hour and annual demand. This was done using FAA Advisory Circular 150/5060-5 *Airport Capacity and Delay*, which uses the factors of aircraft mix index, the runway use configuration, the Airport meteorology, and the percentage of touch-and-go operations to determine these values. The values developed were compared to the long-range forecast for the Airport to determine where any shortfalls exist or may develop.

The ASV was calculated to be 225,000 annual operations. The results of the long-range capacity analysis are presented in Table 4-3. A generally accepted benchmark is to plan for additional runway capacity when demand reaches 60 percent of the ASV. Southwest Oregon Regional Airport's runways, taxiways, and airspace have sufficient capacity to accommodate the expected number of aircraft operations at the Airport without undue delays or congestion.

	<u>Actual</u>		<u>Forecast</u>		
Description	2010	2015	2020	2025	2030
Hourly Operational Capacity					
VFR	76	76	76	76	76
IFR	59	59	59	59	59
Peak Hour Operational Demand	7	7	8	8	8
Hourly Demand/Capacity					
VFR	10%	10%	10%	10%	11%
IFR	13%	12%	13%	13%	14%
Annual Service Volume (ASV)	225,000	225,000	225,000	225,000	225,000
Forecast Annual Operational Demand	13,907	13,513	14,013	14,552	15,133
Annual Demand / ASV	6%	6%	6%	6%	7%

#### Table 4-3 AIRFIELD CAPACITY

Source: RS&H, 2012.



## 4.5.2 Runway Design

The runway analysis addresses the ability of the existing runways at the Airport to accommodate the current and forecast demand. At a minimum, runways must have the proper length, width, and strength to meet FAA recommended design standards to safely accommodate the design aircraft. This section analyzes specific runway criteria and makes recommendations based on the forecast. Elements to be examined in this section include:

- Runway Designation
- Runway Length
- Runway Width
- Runway Protection Zone.
- Runway Geometric and Separation Standards

## 4.5.2.1 Runway Designation

Runway designations provided on each runway indicate the runway orientation according to the magnetic azimuth. Runway designations do change over time. This is due to the slow drift of the magnetic poles on the Earth's surface; however, the runways stay fixed and the magnetic bearing will change. Depending on an airport's location and how much drift takes place, it may be necessary over time to change the runway designation. The true bearing information, shown in Table 4-4 for all runways, is obtained from actual survey data. The runway magnetic azimuths for Runways 04/22 and 13/31 are within tolerance for magnetic azimuth, the runways are not in need of a re-designation.

Runway	True Bearing	Magnetic Variation	Runway Magnetic Azimuth
Runway 04	60° 39' 36.00"	15 <sup>°</sup> 46' 00" East	44° 53' 36.00"
Runway 22	240 <sup>°</sup> 40' 12.00"	15 <sup>°</sup> 46' 00" East	224 <sup>°</sup> 54' 12.00"
Runway 13	149 <sup>°</sup> 37' 48.00"	15 <sup>º</sup> 46' 00" East	133° 51' 48.00"
Runway 31	329° 38' 24.00"	15 <sup>º</sup> 46' 00" East	313° 52' 24.00"

Table 4-4
TRUE RUNWAY BEARING

Sources: National Climatic Data Center and Survey Data, FAA Aeronautical Data Support, FAA National Aeronautical Navigational Services, 2012.

### 4.5.2.2 Runway Length

Runway length is determined by the greater of the takeoff or landing performance characteristics of the existing and future critical aircraft operating at Southwest Oregon Regional Airport, or composite family of airplanes as represented by the critical aircraft's Airport Reference Code (ARC). The takeoff length, including takeoff run, takeoff distance, and accelerate-stop distance, is typically the more demanding of the runway length requirements. As described below, there are two primary means for determining the Airport's recommended runway lengths:



- Guidance A FAA Recommended Runway Length: General runway length guidance based on FAA computer modeling software and Advisory Circular performance graphs for composite aircraft groups, as adjusted for Southwest Oregon Regional Airport mean maximum temperature (66°F), field elevation (17 feet above mean sea level), difference in runway end elevations (± 3 feet to 4 feet), and aircraft flight range of greater than 500 nautical miles.
- Guidance B Critical Aircraft Planning Manual (Performance Curves): Determines runway length for specific aircraft models and engines based on data from the aircraft manufacturer, as adjusted for Southwest Oregon Regional Airport to the extent possible based on aircraft operating (payload) weights, flight range, non-standard temperatures, and field elevation (17 feet above mean sea level).

Based on the existing primary runway length of 6,000 feet (for takeoffs), the forecast aircraft operations, and expected stage length, Guidance B was used for further evaluation to determine if additional runway length is needed during the planning period.

As discussed in **Chapter 3, Aviation Demand Forecasts**, the future critical aircraft for the Airport is the Bombardier CRJ-200. The CRJ-200 performance curves were used to determine required runway lengths at specified takeoff weights and at varying temperature levels. Table 4-5 provides the runway length requirements based on CRJ-200 takeoff requirements. Items shown in blue indicate the Maximum Takeoff Weight (MTOW) for a CRJ-200 on the existing 6,000-foot runway.



	Resulting Cond	litions for CRJ-200
		ng Conditions with keoff Runway Length
		Takeoff Runway
Temperature	MTOW (lbs)	Length (ft)
	53,000	6,300
	52,000	6,100
	51,500	6,000
Standard Day (15°C) / 59°F	51,000	5,800
	50,000	5,600
	49,000	5,400
	48,000	5,180
	53,000	6,450
Standard Day + 8°C / 73°F	52,000	6,200
	51,000	6,000
	50,000	5,700
	49,000	5,500
	48,000	5,200
	53,000	7,000
	52,000	6,750
	51,000	6,450
Standard Day + 15°C / 86°F	50,000	6,150
	49,500	6,000
	49,000	5,900
	48,000	4,750
	53,000	7,600
	52,000	7,000
Standard Day + 20°C / 95°F	51,000	6,900
Stanuaru Day + 20 C / 95 F	50,000	6,580
	49,000	6,300
	48,000	6,000
	MLW (lbs)	Landing Runway Length (ft)
Standard Day (15°C) / 59°F	47,000	5,635

# Table 4-5 BOMBARDIER CRJ-200 RUNWAY LENGTH REQUIREMENTS

Source: RS&H Analysis, 2012. Bombardier, Inc. Airport Planning Manual, Issue 7, Model CL-600-2B19 Series 100/200/440, 2003.

Note: Runway lengths are based upon wet runway conditions at sea level. Controlling obstructions are not included in the analysis. Maximum Takeoff Weight (MTOW), Maximum Landing Weight (MLW).



Table 4-5 indicates that the CRJ-200 can takeoff the existing 6,000-foot runway on a Standard Day (59°F) at a MTOW of 51,500 pounds, 51,000 pounds at 73°F, 49,500 pounds at 86°F, and a MTOW of 48,000 pounds at 95°F.

As discussed in **Chapter 3**, **Aviation Demand Forecasts –** *Alternative Forecast Scenario*, the alternative forecast scenario considers the return of a second major airline for the long-term with flights to additional hubs, including Salt Lake City International Airport (SLC) and Denver International Airport (DEN). Further analysis was conducted to define the maximum allowable range (in nautical miles) within these parameters in order to determine if additional runway length would be required to reach these hubs, as well as the additional destinations below:

- Seattle-Tacoma International Airport (SEA)
- Los Angeles International Airport (LAX)
- Dallas/Fort Worth International Airport (DFW)
- Chicago Midway International (MDW)
- Orlando International Airport (MCO)

This analysis is shown in Table 4-6 depicts CRJ-200 payload, range, and MTOW results. Specified flight path for the destinations listed above are shown in blue next to their corresponding range (in nautical miles). Items in **bold** indicate the factor held constant for the equation used to determine payload and range.



Resulting Conditions for Payload & Range							
		% of Useable Payload				Estimated	
OEW (Ibs)	MZFW (Ibs)	Fuel Loaded	(lbs)	(%)	MTOW (lbs)	Range (NMs) <sup>(1)</sup>	Destination
30,500	44,000	63%	13,500	100%	53,000	1,140	
30,500	42,650	72%	12,150	90%	53,000	1,460	
30,500	41,300	82%	10,800	80%	53,000	1,700	
30,500	39,950	91%	9,450	70%	53,000	1,970	
30,500	38,600	100%	8,100	60%	53,000	2,370	
30,500	44,000	63%	13,500	100%	53,000	1,140	
30,500	44,000	56%	13,500	100%	52,000	1,050	
30,500	44,000	49%	13,500	100%	51,000	860	
30,500	44,000	42%	13,500	1 <b>00</b> %	50,000	750	
30,500	44,000	35%	13,500	100%	49,000	570	
30,500	44,000	28%	13,500	100%	48,000	390	
30,500	44,000	24%	13,500	100%	47,450	255	OTH - SEA
30,500	44,000	35%	13,500	100%	49,000	570	OTH - SLC
30,500	44,000	38%	13,500	100%	49,500	630	OTH - LAX
30,500	44,000	52%	13,500	100%	51,500	904	OTH - DEN
30,500	43,500	66%	13,000	96%	53,000	1,425	OTH - DFW
30,500	42,500	73%	12,000	89%	53,000	1,607	OTH - MDW
30,500	39,500	94%	9,000	67%	53,000	2,247	OTH - MCO

 Table 4-6

 BOMBARDIER CRJ-200 PAYLOAD AND RANGE ANALYSIS

Sources: RS&H Analysis, 2012. Bombardier, Inc. Airport Planning Manual, Issue 7, Model CL-600-2B19 Series 100/200/440, 2003. Flight path distances obtained from <u>www.gcmap.com</u>.

<sup>(1)</sup> Payload/Range for Mach 0.80 Cruise at 37,000 feet, ISA conditions (59°F), zero winds, 50 passengers and bags at 200 pounds. Includes fuel reserves (10 minute taxi allowance, hold 45 minutes at cruising altitude, 100 NM alternate, fuel density = 6.7 pounds per US gallon.

Notes: Operating Empty Weight (OEW), Maximum Takeoff Weight (MTOW), Maximum Design Zero Fuel Weight (MZFW), Nautical Miles (NMs). MZFW is the result of OEW + Payload. Items in **bold** indicate the factor held constant for the equation.

The results of this analysis determined that the CRJ-200 could takeoff from Southwest Oregon Regional Airport with a MTOW of 51,500 pounds and fly to Denver International Airport (904 nautical miles) on a standard day (59°F) at 100% payload (total weight of passengers, baggage, and cargo). On a hot day (95°F), the CRJ-200 could take off with 48,000 pounds MTOW, with 100% payload and fly to Seattle/Tacoma International Airport (255 nautical miles).

The planning standard of  $73^{\circ}$ F is used to represent typical peak seasonal temperatures during the summer months. Table 4-7 below provides the achievable ranges and the corresponding payload levels with the runway takeoff length required on a standard day + 15°C (73°F).



		% of Useable	Payload			Estimated
OEW (Ibs)	MZFW (Ibs)	Fuel Loaded	(lbs)	(%)	MTOW (lbs)	Range (NMs) <sup>(1)</sup>
30,500	44,000	49%	13,500	100%	51,000	860
30,500	42,650	58%	12,150	90%	51,000	1,200
30,500	41,300	68%	10,800	80%	51,000	1,670
30,500	39,950	77%	9,450	70%	51,000	1,830
30,500	38,600	87%	8,100	60%	51,000	2,130

Table 4-7				
PEAK SEASON TEMPERATURE RUNWAY TAKEOFF LENGTH REQUIREMENTS				

Sources: RS&H Analysis, 2012. Bombardier, Inc. Airport Planning Manual, Issue 7, Model CL-600-2B19 Series 100/200/440, 2003. Flight path distances obtained from <u>www.gcmap.com</u>.

<sup>(1)</sup> Payload/Range for Mach 0.80 Cruise at 37,000 feet, ISA conditions (59°F), zero winds, 50 passengers and bags at 200 pounds. Includes fuel reserves (10 minute taxi allowance, hold 45 minutes at cruising altitude, 100 NM alternate, fuel density = 6.7 pounds per US gallon.

Notes: Operating Empty Weight (OEW), Maximum Takeoff Weight (MTOW), Maximum Design Zero Fuel Weight (MZFW), Nautical Miles (NMs). MZFW is the result of OEW + Payload. Items in **bold** indicate the factor held constant for the equation.

On a peak season day, the CRJ-200 could takeoff from Southwest Oregon Regional Airport with 90% payload and fly a range of approximately 1,200 nautical miles, exceeding the distance to Denver International Airport (904 nautical miles).

Table 4-8 below provides a summary of the CRJ-200's payload and ranges achievable at varying temperatures, based on the limitations of the existing takeoff runway length. During peak season temperatures, the maximum range possible from Southwest Oregon Regional Airport with 100% payload is approximately 860 nautical miles.

Existing Takeoff Runway Length (ft)	Temperature	Payload (%)	MTOW (lbs)	Estimated Range (NMs)
6,000	59°F	100%	51,500	904
6,000	73°F	100%	51,000	860
6,000	86°F	100%	49,500	630
6,000	95°F	100%	48,000	390

# Table 4-8 SUMMARY OF EXISTING CRJ-200 OPERATIONAL LIMITATIONS

Sources: RS&H Analysis, 2012. Bombardier, Inc. Airport Planning Manual, Issue 7, Model CL-600-2B19 Series 100/200/440, 2003.

Note: Maximum Takeoff Weight (MTOW), Nautical Miles (NMs).



It is important to note that the runway length analysis above does not include limitations that may be caused by controlling obstructions. Controlling obstructions within the operating environment may limit takeoff weights necessary to maintain a minimum climb rate that would clear the obstruction. However, this analysis was necessary to determine if the CRJ-200 would require additional length to reach eastbound destinations, such as Denver, with as much payload as possible. This <u>initial</u> analysis indicates that the existing runway takeoff length is adequate for the CRJ-200 to reach Denver International Airport during the peak season with 90% payload.

Due to the Airport's immediate proximity to the Port of Coos Bay, an additional detailed analysis was conducted to evaluate if controlling obstructions, specifically "ship conditions" for large shipping vessels navigating the Port of Coos Bay, would have an impact on the payload and range of the Bombardier CRJ-200 during peak season conditions. For example purposes only, runway extensions of 200 feet and 400 feet on both ends of Runway 04/22 were also reviewed. This additional evaluation is based on CRJ-200 takeoff weight data for Southwest Oregon Regional Airport, as performed by Jeppesen<sup>1</sup>. Table 4-9 below provides the results of the Jeppesen analysis combined with Bombardier CRJ-200 Performance Curve data in order to reach a minimum range of 910 nautical miles (for a Denver route) during the peak season.

The Port of Coos Bay shipping channel curves around Runway 04/22, approximately 3,520 feet west from the Runway 04 departure end and approximately 1,000 feet north/northeast of the Runway 22 departure end (as published in the May 2012 FAA Airport Facility Directory). Shipping vessels within this channel can have mast heights up to 140 feet, which then become the controlling obstacle when the shipping channel is active. When ship conditions are not active as the controlling obstruction, the CRJ-200 can achieve 85 percent payload on the existing 6,000-foot takeoff runway length. This is due to field limitations because the aircraft must be able to accelerate for takeoff and bring the aircraft to a stop within the existing runway length. However, without the presence of obstructing ships, extending the runway 400 feet would increase payload roughly ten percent for a Denver route during peak season temperatures. This payload increase equates to approximately 7.5 additional seats<sup>2</sup> gained.

The analysis shown in Table 4-9 indicates that the existing 6,000 foot Runway 22 takeoff length is sufficient for a CRJ-200 to reach Denver International Airport during peak season temperatures and ship conditions with 80 percent payload. However, regardless of additional length, payload will not exceed 80 percent for a Denver route in peak season temperatures when large shipping vessels are the controlling obstruction. This performance limitation occurs because the aircraft must be light enough to depart the runway in order to safely clear the obstacle by 35 feet, while having enough length to abort the takeoff and bring the aircraft to a stop. Therefore, whenever a ship condition is present, additional runway length would not increase payload yield during ship conditions.

Although Runway 22 is the primary takeoff runway, Runway 04 (including 200 and 400-foot extensions) was also analyzed for performance with and without ship conditions. Table 4-9 shows that with a 400-foot extension, takeoffs from Runway 04 are limited to an 84 percent payload without ship conditions and 59 percent payload with ship conditions. This is due to the closer proximity of the shipping channel as the controlling obstacle. The additional pavement would



<sup>&</sup>lt;sup>1</sup> Jeppesen Flight Operations Division, Takeoff Weight Chart, for Bombardier CL-600-2B19, Airplane Flight Manual (AFM) Revision 54, Southwest Oregon Regional Airport, May 2012.

<sup>&</sup>lt;sup>2</sup> Estimate is based on Bombardier CRJ-200 Airplane Planning Manual parameters of 200 pounds per passenger for 50 passengers and bags.

increase the ability of Runway 04 to accommodate heavier CRJ-200 traffic in non-ship conditions; however, the payload yield is not optimal for a Denver route.

	Takeoff Runway			% of Useable	Pay	oad	
Runway Condition	Length	OEW (lbs)	MZFW (lbs)	Fuel Loaded	(lbs)	(%)	MTOW (lbs)
Runway 22 - Existing Condition	6,000	30,500	42,000	52%	11,500	85%	49,500
Runway 22 - Ship Condition	6,000	30,500	41,300	47%	10,800	80%	48,000
Runway 22 - 200' Extension	6,200	30,500	43,000	51%	12,500	93%	50,300
Runway 22 - 200' Extension, Ship Condition	6,200	30,500	41,300	47%	10,800	80%	48,000
Runway 22 - 400' Extension	6,400	30,500	43,400	53%	12,900	96%	51,000
Runway 22 - 400' Extension, Ship Condition	6,400	30,500	41,300	47%	10,800	80%	48,000
Runway 04 - Existing Condition	5,321	30,500	39,950	48%	9,450	70%	46,800
Runway 04 - Ship Condition	5,321	30,500	37,655	35%	7,155	53%	42,600
Runway 04 - 200' Extension	5,521	30,500	40,895	48%	10,395	77%	47,700
Runway 04 - 200' Extension, Ship Condition	5,521	30,500	38,060	35%	7,560	56%	43,100
Runway 04 - 400' Extension	5,721	30,500	41,840	45%	11,340	84%	48,300
Runway 04 - 400' Extension, Ship Condition	5,721	30,500	38,465	37%	7,965	59%	43,700

Table 4-9
CRJ-200 RUNWAY CONDITIONS & RESULTS FOR A RANGE OF 910 NAUTICAL MILES AT 72° F

Sources: RS&H Analysis, 2012; Jeppesen Flight Operations Division, Takeoff Weight Chart, CL-600-2B19 CF 34-B1 AFM Revision 54 for KOTH, May 2012; and Bombardier, Inc. Airport Planning Manual, Issue 7, Model CL-600-2B19 Series 100/200/440, 2003.

Notes: Payload/Range for Mach 0.80 Cruise at 37,000 feet. Operating Empty Weight (OEW), Maximum Takeoff Weight (MTOW), Maximum Design Zero Fuel Weight (MZFW).

Both analyses concluded that when ship conditions exist, the CRJ-200 can perform well on the existing runway for short stage length markets such as Los Angeles (94 percent payload), Salt Lake City (96 percent payload), and Seattle (100 percent payload) during peak season temperatures. The runway length analyses also concluded that to achieve a viable payload yield for a CRJ -200 on a midrange market such as Denver, a 400 foot runway extension on Runway 22 would be necessary. The 400 foot extension would increase payloads to 95% during peak season temperatures. Without an extension, payloads would drop to unacceptable levels when conditions were outside of the standard conditions. Airline operators utilizing the CRJ-200 are unlikely to add a Denver route with existing runway lengths because payload yields may not be profitable.

### 4.5.2.3 <u>Runway Widths</u>

Both Runways 04/22 and 13/31 have a width of 150 feet. This width exceeds the design standard and is sufficient for the existing and future critical aircraft for both runways. The most recently approved Airport Layout Plan (2005) indicates that in 2003, a waiver to maintain Runway 13/31's width at 150 feet was granted by the FAA Seattle Airports District Office, which remains in effect until such time as the runway is reconstructed.

#### 4.5.2.4 Runway Protection Zone

For the protection of people and property on the ground, the FAA has identified an area of land off each runway end as the runway protection zone (RPZ). The trapezoidal-shaped RPZ is centered on the extended runway centerline starting 200 feet from the paved runway end. The RPZ varies in width and length based on runway instrument approach classification. The FAA recommends that the Airport own or control the entire RPZ, through property deeds and/or avigation easements.



Existing and Future RPZ dimensions are provided in Table 4-10. The following changes are expected to occur within the 20-year planning period:

• In conjunction with the primary runway ARC upgrade to C-III, the Runway 22 RPZ is expected to accommodate Category C aircraft, which will result with an increase in length, outer width, and acreage.

Currently, the Airport does not own or control the entire RPZ for all runway ends. This includes approximately 22 acres in the existing Runway 04 RPZ, 0.3 acres in the existing Runway 22 RPZ, five acres in the future Runway 22 RPZ, two acres in the Runway 13 RPZ, and approximately nine acres in the Runway 31 RPZ. The necessity for acquisition of additional easement will be assessed as part of the airfield alternative analysis.

RUNWAY PROTECTION ZONE DIMENSIONS								
	<u>Runw</u>	unway 04 <u>Runway 22 Runway 13</u>		<u>Runway 22</u>		<u>Runway 31</u>		
Runway	Existing	Future	Existing	Future	Existing	Future	Existing	Future
Length	2,500'	2,500'	1,000'	1,700'	1,000'	1,000'	1,000'	1,000'
Inner Width	1,000'	1,000'	500'	500'	500'	500'	500'	500'
Outer Width	1,750'	1,750'	700'	1,010'	700'	700'	700'	700'
Acreage	78.91	78.91	13.77	29.47	13.77	13.77	13.77	13.77

Table 4-10RUNWAY PROTECTION ZONE DIMENSIONS

Sources: FAA Advisory Circular 150/5300-13 and Airport Records.

#### 4.5.2.5 Runway Geometric and Separation Standards

This section analyzes the existing runway geometric and separation distances against the dimensional standards that arise from the critical aircraft category designated for each runway. Compliance with FAA airport geometric and separation standards, without modification to standards, is intended to meet a minimum level of airport operational safety and efficiency.

Table 4-11 compares the FAA airport design standards for both runways, based on the existing and future Airport Reference Code. As shown below, the Runway 22 Object Free Area (OFA) length currently does not comply with either B-III or C-III design standards. Approximately 0.5 acres of the existing and future OFA for Runway 22 is not entirely contained on airport property. Similarly, Runway 31 also has a nonstandard OFA, as it is approximately 380 feet short of the B-III OFA length requirement. As indicated in FAA Advisory Circular (AC) 150/5300-13, Chapter 2, existing and planned airport elements, including Object Free Areas, should be contained on airport property. It is also important to note that a portion of the East Perimeter Road is located within the Runway 13/31 OFA, south of the Runway 31 threshold. The Alternatives Analysis will examine feasible solutions that incorporate compliance with runway design standards, which will be compounded with AC 150/5300-13A.



	<u>R</u>	unway 04/22 (Pri	<u>Runway 13/</u>	<u>31 (Crosswind)</u>	
	Existing Dimension	ARC B-III <sup>(1)</sup>	ARC C-III <sup>(1)</sup>	Existing Dimension	ARC B-III (2)
Runway Design Standards					
Runway Width	150'	100'	100'	150'	100'
Runway Shoulder Width	N/A	20'	20'	N/A	20'
Runway Blast Pad Width	150'	140'	140'	150'	140'
Runway Blast Pad Length	250'	200'	200'	424'	200'
Runway Safety Area (RSA) Width	500'	400'	500'	300'	300'
Runway Safety Area (RSA) Length					
Prior to Landing Threshold	600' / 600'	600' / 600'	600' / 600'	600' / 600'	600' / 600'
Runway Safety Area (RSA) Length				100000000000000000000000000000000000000	
Beyond Runway End	1,000' / <b>899'</b>	800' / 800'	1,000' / 1,000'	600' / 600'	600' / 600'
Runway Object Free Area (OFA) Width	800'	800'	800'	800'	800'
Runway Object Free Area (OFA) Length					
Beyond Runway End	1,000' / <b>604'</b>	800' / 800'	1,000' / 1,000'	643' / <b>220'</b>	600' / 600'
Runway Obstacle Free Zone (OFZ) Width	400'	400'	400'	400'	400'
Runway Obstacle Free Zone (OFZ) Length					
Beyond Runway End	200' / 200'	200' / 200'	200' / 200'	444' / 200'	200' / 200'
Precision Obstacle Free Zone (OFZ) Width	800'	800'	800'	N/A	800'
Precision Obstacle Free Zone (OFZ)					
Length Beyond Runway End	200' / N/A	200' / 200'	200' / 200'	N/A	200' / 200'
Runway Centerline Separation Standards					
Runway to Parallel Taxiway	400'	350'	400'	325'	300'
Runway to Holdline	250'	250'	250'	250'	200'
Runway to Aircraft Parking Area	658'	400'	500'	400'	250'

 Table 4-11

 RUNWAY GEOMETRIC AND SEPARATION STANDARDS

Sources: FAA Advisory Circular 150/5300-13, Airport Design. Existing dimensions obtained from the current OTH Airport Layout Plan.

Note: Items in red, bolded text reflect potential non-standard criteria.

<sup>(1)</sup> With lower than <sup>3</sup>/<sub>4</sub> mile approach visibility minimums. Existing runway approach visibility minimums = <sup>1</sup>/<sub>2</sub>-mile to Runway 04 only.

<sup>(2)</sup> With visual and not lower than <sup>3</sup>/<sub>4</sub> mile approach visibility minimums.

As shown in Table 4-11, the Runway 22 Runway Safety Area does not meet the RSA length requirement for C-III. Based on airfield site evaluation made by the FAA during a recent Part 139 Certification Inspection, it was determined that the water intrudes into the RSA. The 2010 revised Airport Layout Plan indicates that the Runway 22 RSA length extends 899 feet before the land curves into the water, and is 101 feet short of the full 1,000-foot RSA length design standard for ARC C-III. In order to upgrade Runway 04/22, the RSA length must be extended to meet the design standard. Solutions to address the future C-III RSA length for Runway 22 will be evaluated in **Chapter 5, Alternatives Analysis**.



## 4.5.2.6 Modifications to Standards

The previous ALP identified two airport design issues that required modifications to airport design standards. Coordination with the FAA during the Facility Requirements analysis has initially determined that no official documentation of the original approval has been identified at this time, and that these are likely a "deviation to design standards". Further FAA coordination for official approval or alternatives to address these design issues will be presented in **Chapter 5**, **Alternatives Analysis**. Table 4-12 provides a description of these "deviations to design standards".

Runway	Description of Modifications
Runway 13/31	Existing/ultimate OFA is not available due to property restrictions, perimeter fence, and bodies of water.
Runway 04/22	The ultimate OFA is not available due to property restrictions and bodies of water.

 Table 4-12

 EXISTING MODIFICATIONS TO AIRPORT DESIGN STANDARDS

Source: OTH Airport Layout Plan, December 2010 revision.

## 4.5.3 <u>Taxiway Design</u>

This taxiway analysis addresses specific requirements relative to FAA design criteria and the ability of the existing taxiways to accommodate the current and projected demand. At a minimum, taxiways must provide efficient circulation, must have the proper strength, and meet recommended FAA design standards to safely accommodate the design aircraft. Taxiway geometry, separation distance between taxiways, and taxiway widths all contribute to the ability of aircraft to taxi around the airfield. Airport runways should be supported by a system of taxiways that provides an access interface between the runways and the aircraft parking and hangar areas.

The Airport's design aircraft determines taxiway design standards and dimensional criteria. Since Southwest Oregon Regional Airport will remain Airplane Design Group (ADG) III (due to peak operations conducted by ARC A/B/C/D-III aircraft), it is recommended that critical airfield taxiways be maintained and/or designed and built to the standard FAA ADG-III taxiway design standards.

Depending upon the demand, portions of an airfield may be designed for one aircraft type and other portions for a different aircraft type. At Southwest Oregon Regional Airport, all of the taxiways serving the runways, should meet the recommended design standards for ADG-III. The design requirements for taxiways (J and K, located west of Runway 31) primarily intended to serve small aircraft, need only meet design standards for ADG-II. The FAA recommended design standards for taxiways and taxilanes for the appropriate ADG are provided in

Table 4-13 along with the existing geometry and safety separation of each taxiway on the Airport.

Taxiway B, which is located at the northern edge of the main apron, has a non-standard taxiway width of 42 feet. Further, Taxiways A, B, C, D, F, and G currently do not have stabilized or paved shoulders, as required in FAA AC 150/5300-13 for taxiways accommodating Airplane Design Group III aircraft. This design standard will also be carried forward in AC 150/5300-13A.



	<u>Taxiway</u>							
Airfield Component	ADG-III	Α	В	С	D	F	G	
Taxiway Design Standards								
Taxiway Width	50'	50'	42'	50'	80'	75'	75'	
Taxiway Shoulder Width	20'	0'	0'	0'	0'	0'	0'	
Taxiway Safety Area Width	118'	118'	118'	118'	118'	118'	118'	
Taxiway Object Free Area Width	186'	186'	186'	186'	186'	186'	186'	
Taxilane Object Free Area Width	162'	N/A	N/A	N/A	N/A	N/A	N/A	
Taxiway Centerline Separation Standards	;							
Runway to Parallel Taxiway	350'	325' <sup>(1)</sup>	N/A	400'	N/A	N/A	N/A	
Taxiway to Parallel Taxiway/Taxilane	152'	152'	289'	300'	N/A	N/A	N/A	
Taxiway to Fixed or Moveable Object	93'	93'	93'	93'	93'	93'	93'	

Table 4-13 TAXIWAY GEOMETRIC AND SEPARATION STANDARDS

		Tax	iway	
Airfield Component	ADG-II	J <sup>(2)</sup>	K <sup>(2)</sup>	
Taxiway Design Standards				
Taxiway Width	35'	35'	35'	
Taxiway Shoulder Width	10'	N/A <sup>(3)</sup>	N/A <sup>(3)</sup>	
Taxiway Safety Area Width	79'	79'	79'	
Taxiway Object Free Area Width	131'	131'	131'	
Taxilane Object Free Area Width	115'	N/A	N/A	
Taxiway Centerline Separation Standards				
Runway to Parallel Taxiway	240'	N/A	N/A	
Taxiway to Parallel Taxiway/Taxilane	105'	N/A	N/A	
Taxiway to Fixed or Moveable Object	65.5'	66'	66'	

Source: FAA Advisory Circular 150/5300-13, Airport Design.

Notes: Items in red, bolded text reflect potential non-standard criteria. Taxiway H was not included due to its exclusive use for US Coast Guard operations.

<sup>(1)</sup> The runway to parallel taxiway centerline separation is 300 feet for a B-III with visual or not lower than 3/4-mile visibility minimums (existing/future Runway 13-31).

<sup>(2)</sup> Taxiways J and K are designed to meet Airplane Design Group II standards.

<sup>(3)</sup> Stabilized or paved shoulders are required for taxiways that will accommodate Airplane Design Group III or higher, per FAA AC 150/5300-13, Change 17.

### 4.5.3.1 Taxiway Intersections

As shown previously on Figure 4-4 as "H-1", the FAA Airport Diagram indicates H-1 as a hot spot at Southwest Oregon Regional Airport, located at the intersection of Taxiway C and the north end of Runway 13/31. The FAA reports that pilots have mistaken the runway as part of Taxiway C and taxied onto Runway 13/31 without clearance. The FAA defines Hot Spots as intersections that are confusing to pilots in ways that may lead to runway incursions. Often, hot spots have a complex or confusing taxiway-runway intersection or taxiway-taxiway intersection. Many hot spots are taxiway intersections that are incompatible with the "3-Node principle". As stated in FAA AC 5300/150-13, Change 17, all existing taxiway intersections designated as a hot spot "are to be constructed in accordance with the "3-Node" design principle". The 3-Node design principle is a taxiway intersection with no more than three directions to proceed beyond the intersection, which allows pilots to continue through the intersection or turn left or right. The 3-Node design helps minimize runway incursions, reduce confusion for pilots, and result in improved compliance with markings, airfield lighting, and signage instructions.



The intersection of Taxiway A and Taxiway C occurs 325 feet west of the Runway 13 threshold. For Runway 22 departures, aircraft taxiing north on Taxiway A with the intent to turn right onto Taxiway C will not encounter the markings and signage for the Runway 13 intersection until the aircraft is already turning onto Taxiway C. Depending upon aircraft size, when making the right turn at this intersection, the aircraft may exceed the holdline and inadvertently cross Runway 13. Draft AC 150/5300-13A provides guidance to improve existing taxiway design, with an emphasis on hot spots; therefore, this issue would exist when the final AC 150/5300-13A is published. Alternatives to improve the situational awareness and prevent potential runway incursions in this hot spot area will be examined in **Chapter 5, Alternatives Analysis**.

Additional FAA guidance regarding runway incursion prevention is published in Engineering Brief (EB) No. 75: Incorporation of Runway Incursion Prevention into Taxiway and Apron Design. EB No. 75 suggests that taxiway layouts providing straight, direct access from a terminal or parking area onto a runway should be avoided, and especially those located within the midsection of the runway. Currently, Taxiway A1 and Taxiway A2 provide direct, straight line access from the main apron onto Runway 13/31. Although EB No. 75 will likely become obsolete once AC 150/5300-13A is published, the Draft AC 150/5300-13A incorporates indirect access between a runway and an aircraft parking area as a design standard. Solutions to address the existing direct access from the main apron to Runway 13/31 via Taxiways A1 and A2 will be evaluated in the next chapter.

## 4.5.4 Pavement Strength & Condition

Pavement strength is an important criterion in determining the usability of the airfield. Table 4-14 lists the weights of the more demanding aircraft currently using or expected to use Southwest Oregon Regional Airport. Small and medium general aviation business jets typically range between 12,000 to 50,000 pounds. Those that have a maximum takeoff weight of more than 20,000 pounds have a dual-wheel gear (DWG) configuration. Air carrier turboprop aircraft typically range from 22,000 to 65,000 pounds DWG, and regional jet aircraft range from 40,000 to 55,000 pounds DWG.

Aircraft	Aircraft Size (Passengers)	ARC	Gear Type	Maximum Take-Off Weight
General Aviation Aircraft				
Light/Small Business Jet	4 to 6 Passengers	B-I to B-II	Single-Wheel	8,000 to 20,000 lbs.
Medium Business Jet	6 to 10 Passengers	B-II to C-II	Dual-Wheel	20,000 to 50,000 lbs.
Large Business Jet	10 to 16 Passengers	C-II to D-III	Dual-Wheel	45,000 to 95,000 lbs.
Boeing Business Jet (737-700)	126 to 150 Passengers	C-III	Dual Tandem-Wheel	155,000 to 171,000 lbs
Air Carrier Aircraft				
Turboprop	19 to 40 Passengers	B-II to A-III	Dual-Wheel	22,000 to 65,000 lbs.
Regional Jet	50 to 90 Passengers	C-II	Dual-Wheel	40,000 to 83,000 lbs.

#### Table 4-14 AIRCRAFT WEIGHTS

Source: Reynolds, Smith and Hills, Inc., 2012.



Table 4-15 identifies recommended runways, taxiways, and apron pavement strengths at the Airport. Currently, the Airport does not have a Pavement Management Program, which is used to make decisions on timing and the type of maintenance necessary for airfield pavement. All existing runways and taxiways meet the recommended pavement strengths; however, the existing pavement strength for the apron areas is unknown. A Pavement Management Study is recommended in order determine accurate pavement strengths of the apron areas and to assist the timing and maintenance necessary to preserve airfield pavement longevity.

Pavement Area	Existing Pavement Strength (Gear Type)	Required Pavement Strength (Gear Type)
Bupuev 04/22	106, 000 lbs. (SWG)	90, 000 lbs. (SWG)
Runway 04/22	113,000 lbs. (DWG)	113,000 lbs. (DWG)
& Parallel Taxiway System	190,000 lbs. (DTW)	190,000 lbs. (DTW)
Bupwoy 12/21	90, 000 lbs. (SWG)	90, 000 lbs. (SWG)
Runway 13/31	100,000 lbs. (DWG)	100,000 lbs. (DWG)
& Parallel Taxiway System	100,000 lbs. (DTW)	100,000 lbs. (DTW)
Apron (Air Carrier)	UNKNOWN	65,000 lbs. (DWG)
Apron (Main GA)	UNKNOWN	95,000 lbs. (DWG)
Apron (South GA)	UNKNOWN	20,000 lbs. (SWG)

# Table 4-15REQUIRED PAVEMENT STRENGTHS

Sources: FAA Advisory Circular 150/5320-6e and Airport Records, 2012.



## 4.6 AIRSPACE, NAVIGATIONAL AND VISUAL AID REQUIREMENTS

The national airspace system consists of various classifications of airspace that are regulated by the FAA. Airspace classification is necessary to ensure the safety of all aircraft utilizing the facilities, particularly during periods of inclement weather. Navigational and visual aids consist of equipment that helps pilots locate the Airport and provides information to pilots about the aircraft's horizontal alignment, height above the ground, the location of airport facilities, and the aircraft's position on the airfield.

## 4.6.1 <u>Airspace Requirements</u>

The current national airspace classification for Southwest Oregon Regional Airport is Class D. A detailed description of the National Airspace System and all airspace classifications are provided in **Appendix D, National Airspace System.** The Class D airspace classification is sufficient for the existing and future operational requirements expected at the Airport.

Air traffic in the vicinity of the Airport is controlled by the North Bend Airport Traffic Control Tower (ATCT). The ATCT controllers maintain all air-to-ground communications and visual signaling within five nautical miles and up to 2,500 feet mean sea level (MSL) above the Airport. Additionally, these controllers are responsible for directing ground movement of all aircraft and vehicles within designated portions of the airport that include the runway and taxiway system.

Instrument arrivals and departures are normally controlled by the Seattle Air Route Traffic Control Center (ARTCC). When the ATCT is not in operation from 5:01 pm to 7:59 am, Class E airspace applies and is adequate for the existing and future operational requirements.

Both the City of North Bend and Coos County currently have airspace protection ordinances in place, which are established as the Airport Overlay Zone (Ord. 1952 § 1(4), 2006) in Chapter 18 of the North Bend Municipal Code, and the Air Surface Protection Area, [Ord. 93 § 3.20.1, 1987] in the Coos Bay Municipal Code.

## 4.6.2 Navigational Aids

Navigational aids (NAVAIDs) consist of equipment that helps pilots locate the Airport, provide horizontal guidance information for a non-precision approach, and provide horizontal and vertical guidance information for a precision instrument approach. All of the existing runways at the Airport have appropriate NAVAIDs that are sited correctly and in working condition.

## 4.6.3 Visual Aids

Visual aids consist of a variety of lighting and marking aids used to guide the pilot both in the air and on the ground. These aids provide pilots visual cues on their horizontal and vertical position by providing data regarding the aircraft's alignment, height, and distance from the Airport. The visual aids at the Airport are in working condition and are only in need of routine maintenance except for the following items:

• Runway Lighting Systems - It is recommended that Runway 04 and Runway 13 have the Visual Approach Slope Indicators (VASI) replaced with Precision Approach Path Indicators



(PAPI), which are already installed on Runway 22 and Runway 31. Runway End Identifier Lights (REIL) are also recommended for Runway 22.

• Taxiway Lighting Systems - It is recommended that taxiway edge reflectors on Taxiway K be replaced with Medium Intensity Taxiway Lights (MITL) to allow for both daytime and nighttime operations.



## 4.7 PASSENGER TERMINAL AREA

The passenger terminal area at Southwest Oregon Regional Airport includes the commercial passenger terminal building and the air carrier apron. These areas are specifically designed to serve passengers utilizing the commercial airline services at the Airport.

## 4.7.1 Passenger Terminal Building

The Southwest Oregon Regional Airport commercial service passenger terminal building officially opened in 2008. The terminal building facility requirements, are presented in Table 4-16, and outline the general overall planning space requirements that are calculated from the projected future peak activity levels. This analysis assumed that commercial turboprop aircraft would be phased out and replaced with the 50-seat CRJ-200 in the short-term. Due to the completion of the new passenger terminal building occurring less than five years ago, the approximate 29,834 square foot passenger terminal building provides adequate total area to accommodate the peak forecast passenger enplanement demand throughout the 20-year planning period.

	Actual		For	ecast	
Description	2010	2015	2020	2025	2030
Annual Passengers (Deplaned + Enplaned)	45,507	49,400	56,800	65,300	75,100
Annual Enplanements	22,585	24,701	28,405	32,661	37,560
Peak Month (August)	2,755	3,143	3,614	4,156	4,779
Average Day (31 Days)	89	101	117	134	154
Total Peak Hour Passengers	52	60	69	79	91
Required SF for Total Peak Hour Passengers	12,586	14,357	16,510	18,983	21,831
Total Existing Terminal Area SF	29,834	29,834	29,834	29,834	29,834
Total Terminal Area SF Surplus / (Deficit)	17,248	15,477	13,324	10,851	8,003
Number of Gates	1	1	1	1	2

Table 4-16 PASSENGER TERMINAL FACILITY REQUIREMENTS

Source: Reynolds, Smith and Hills, Inc., 2012. Note: Square Feet (SF).

## 4.7.2 Air Carrier Apron

The existing air carrier apron is an asphalt pavement section, approximately 24,440 square yards and in good condition. There are no passenger loading bridges, and the aircraft use a power-in/power-out taxiing operation. There are currently two airlines that serve Southwest Oregon Regional Airport, which are United Express (operated by SkyWest Airlines) and SeaPort Airlines. The air carrier apron can accommodate up to two ADG-II aircraft simultaneously, such as the Bombardier CRJ-200 or Embraer EMB-120.



## 4.8 VEHICLE ACCESS AND PARKING REQUIREMENTS

Access to the Airport by vehicle consists of connecting roadways that enable arriving and departing users to enter and exit the landside facilities and parking facilities. All airport access roadways can be characterized as either on-airport or off-airport. In addition, the vehicle parking at the Southwest Oregon Regional Airport is divided into three principal user groups: Public parking, rental car parking, and employee parking. This section analyzes the Airport's vehicle access and parking needs throughout the planning period.

## 4.8.1 Off-Airport Access

Facility requirements for off-airport access involve a determination of capacity levels associated with the primary means to and from the Airport. The primary means of ground travel to Southwest Oregon Regional Airport consists of personal automobiles, taxicabs/shuttle buses, or rental cars. There is no public mass transit available at the Airport. The major regional roadway that serves the Airport is U.S Highway 101 (locally referred to as the Oregon Coast Highway). Vehicle access from Highway 101 to the Airport is provided by Virginia Avenue from the southeast. Direct access to the Airport from Virginia Avenue is Maple Street to the north, which bends west turning into Maple Leaf Lane, and becomes Colorado Avenue after intersecting with West Airport Way. The passenger terminal building is located on Airport Lane, which is north off of Colorado Avenue. Other aviation and non-aviation facilities are accessed via West Airport Way and East Airport Way, north/northeast off of Maple Leaf Lane.

All off-airport roadways are maintained by the City of North Bend, Coos County, and State of Oregon. Capacity of all the off-airport access roads is considered to be adequate throughout the 20-year planning period.

## 4.8.2 <u>On-Airport Access</u>

On-airport access roadways are subdivided into two categories: public and restricted access roadways. Public roads are, as the name indicates, roadways that are available for public use and provide access to general aviation, landside facilities, and commercial services facilities. Restricted access roadways are located on airport property and generally provide access to on-airport facilities, such as navigational aids, perimeter fencing, aprons, and all airside facilities that cannot be accessed by the general public.

### 4.8.2.1 <u>General Public Access</u>

The general public access roadways on airport property vary in orientation to their intended destination. Access to the general aviation, non-aviation, and U.S. Coast Guard facilities is provided by East Airport Way and West Airport Way (off of Maple Leaf Lane), which are two-lane interior streets oriented northwest-southeast. A small loop road (also called Colorado Avenue) connects with East Airport Way and Airport Way, oriented northwest-southeast, and extends north of the East/West Airport Way corridor.

Access to the passenger terminal building and public parking is provided by Airport Lane, which loops around the terminal parking areas and in front of the passenger terminal building. It is an asphalt two-lane road that provides primary access to the terminal area. The two-way, two-lane segment circulates an island at the main entrance and terminates in front of the terminal building.



These on-airport general public access roads are currently adequate to serve demand. As new development occurs in the non-aviation area, it may be necessary to modify these access roads to accommodate new tenants and their specific needs.

#### 4.8.2.2 Wayfinding and Signage

Wayfinding for vehicular access to Southwest Oregon Regional Airport includes both street markings and signage. White airplane symbol markings painted on the pavement of Maple Leaf Lane point vehicular traffic in the direction of the Airport. A large sign identifying Southwest Oregon Regional Airport is located off of Airport Lane within the center island for the turnoff towards the terminal building. Additional wayfinding and signage improvements to the Airport may be beneficial to aid in enhancing the approach to the terminal building from Virginia Avenue.

#### 4.8.2.3 Restricted Access

There are six vehicle access gates along these roadways providing secured vehicle access into the Airport Operations Area (AOA) for tenants and authorized users. These access points are used by airport maintenance and FBO personnel to gain access to machinery and service equipment. The Airport's "East Perimeter Road" is a partial perimeter road (formerly called Pony Point Boat Ramp Road), which starts at Virginia Avenue, running northerly along the eastern edge of airport property, and terminating at the old boat ramp. The East Perimeter Road is located within the Runway 13/31 Runway Object Free Area, south of the Runway 31 threshold, and east of the runway until it bends to the east approximately 570 feet east/northeast of Taxiway A3.

Without a complete perimeter roadway system located outside the Runway Object Free Areas, service vehicle and machinery access to other parts of the airfield are limited. These restricted access roadways have the necessary capacity to accommodate the demand through the planning period; however, alternatives for locating the East Perimeter Road outside of the Runway 13/31 Runway Object Free Area, as well as a more complete perimeter roadway to provide airport maintenance crews a safe and secure means of servicing the Airport, will be examined in **Chapter 5, Alternatives Analysis**.

### 4.8.2.4 Fencing and Gates

The entire airport perimeter is fenced with a standard six-foot high, chain link security fence except for areas adjacent to the water, and west of Taxiway C5. A total of nine secured vehicle access gates are located on the Airport; one is located at the entrance of the East Perimeter Road, two are located near the South General Aviation area, one adjacent to the former passenger terminal building, one on the north and south sides of the main hangar, one providing access to the fuel farm and another providing FBO access, and one secured vehicle access gate located directly north of the waste water treatment plant.

### 4.8.3 Public Parking

Southwest Oregon Regional Airport currently provides surface parking positions in three separate lots for passengers, with vehicle access from Airport Lane. The passenger parking lots provide 173 total positions. The lower lot contains 65 parking positions; the first upper lot (southwest of the lower lot) contains approximately 47 parking positions, and the second upper lot contains approximately 61 parking positions.



Parking demand at an airport is directly related to origination enplanements, so it is important to relate the expected growth over this planning period at approximately 2.6 percent annually, which is a growth from 22,585 passengers in 2010 to 37,560 in 2030.

Parking demand at an airport for each type of parking (Short-Term, Long-Term, Employee, and Rental Cars) is calculated differently for each type of use and the demand for each use. Short-Term parking users typically park their vehicles for three hours or less and account for approximately 80 percent of users, while these vehicle spaces account for only 20 percent of total parking available at the Airport due to the turn-over frequency of these spaces spaces.

Based on the existing demand, and the fact that the three pay parking lots are joined, short-term and long-term parking were combined for this analysis. The parking demand analysis is also based on the assumption that 3.5 parking spaces are needed per 1,000 passenger enplanements. This calculation results in a surplus of 87 parking spaces in 2015, and a surplus of 42 by the end of the 20-year planning period, as shown in Table 4-17.

	<u>Actual</u>		For	<u>Forecast</u>		
Description	2010	2015	2020	2025	2030	
Annual Enplanements	22,585	24,701	28,405	32,661	37,560	
Parking (Short / Long Term)						
Parking spaces	173	86	99	114	131	
Surplus / (Deficit) Parking Spaces	-	87	74	59	42	

Table 4-17
PUBLIC PARKING DEMAND PROJECTIONS

Source: Reynolds, Smith and Hills, Inc., 2012.

In addition to the passenger parking, the former passenger terminal (now main general aviation) parking lot, which is located off East Airport Way, contains approximately 119 spaces. The general aviation overflow lot located along the airfield fence line is primarily utilized by tenants and users of the adjacent former passenger terminal building and private corporate hangars. These general aviation parking lots are adequate to serve expected demand within the planning period.

## 4.8.4 Rental Car Parking

The required rental car demand fluctuates based upon passenger demand and time of year. Historical estimates indicate that approximately 12 percent of annual enplanements utilize rental cars. The historical rate is applied to the peak month average day enplanements to determine the ready/return lot parking space demand. Table 4-18 provides the projected rental car parking demand for the forecast of annual enplanements. The rental car ready/return lot contains a total of 30 spaces for rental cars. The rental car operation currently has sufficient capacity and should remain adequate throughout the planning period to provide customers with acceptable rental car service.



	Actual		For		
Description	2010	2015	2020	2025	2030
Annual Enplanements	22,585	24,701	28,405	32,661	37,560
Peak Month (August)	2,891	2,964	3,409	3,919	4,507
Average Day (31 Days)	93	96	110	126	145
Ready Rental Car Parking					
Required No. Parking Spaces	19	20	21	22	24
No. Surplus / (Deficit) Parking Spaces	11	10	9	8	6

 Table 4-18

 RENTAL CAR PARKING DEMAND PROJECTIONS

Source: Reynolds, Smith and Hills, Inc., 2012.

## 4.8.5 Employee Parking

Airport employees utilize the lower lot south of the terminal building, where five dedicated employee parking spots are located prior to the gate entry into the lower lot. Additional overflow parking for badged employees is also located to the east of the terminal building. Employees parking at the Airport include, but are not limited to: airport staff, TSA employees, rental car employees, and airline employees. The existing employee parking spots is considered adequate to accommodate employee parking demand throughout the planning period.



## 4.9 AVIATION SUPPORT FACILITIES

Support facilities at an airport encompass a broad set of functions that exist to ensure the airport is able to fill its primary role in a safe, and efficient manner. Support facilities at Southwest Oregon Regional Airport that were examined included:

- Air Traffic Control Tower
- Air Cargo Facilities
- Fixed Base Operator
- Aircraft Hangars
- Aircraft Aprons
- Aircraft Rescue and Fire Fighting Equipment and Facility
- Airport Maintenance and Snow Removal Equipment Facility
- Airport Fuel Storage
- Deicing Facilities

## 4.9.1 <u>Airport Traffic Control Tower</u>

The Airport Traffic Control Tower (ATCT) facility was constructed in 2009, and is adequate to meet current and forecast operations. No changes are expected to occur with the surrounding airspace and, unless a significant change to airfield geometry occurs, the ATCT facility meets current FAA standards.

### 4.9.2 <u>Air Cargo Facilities</u>

Air cargo facilities are facilities dedicated to providing air mail and air freight/air express. FedEx is currently the only air cargo operator leasing building space, which is located behind the former passenger terminal building adjacent to the ARFF building. The Airport has minimal air cargo operations with only the 1,700-square yard tie-down area used for the FedEx hard stand. As discussed previously, reserving this area on the apron for air cargo operations was included in the aircraft parking apron requirements. Although it is recommended to maintain the hardstand area on the main apron for air cargo operations, no significant change from present day conditions is expected within the planning period.

### 4.9.3 Fixed Base Operators

The Airport has two Fixed Base Operators (FBOs), Ocean Air Aviation and Coos Aviation, which supply fuel, maintenance, aircraft storage, and other support services for the operators of general aviation aircraft. The 20-year forecasted demand anticipates growth to be within the existing business' ability to provide services.

### 4.9.4 <u>Hangars</u>

The quantity of general aviation hangar space required at an airport depends on the total number of based aircraft, but also the local weather conditions, aircraft fleet mix, airport security, and user preference. Operators of single-engine aircraft and light twins are likely to opt for T-hangars or



small box hangars, while general aviation business operations typically utilize corporate and conventional hangars.

Due to the coastal location of the Airport, all 51 based aircraft owners prefer hangars to outdoor tiedown space. As such, all based aircraft are currently stored in hangars. There are nine hangar buildings at the Airport, three of which are owned by the Coos County Airport District, two are owned by the FBOs, and four are privately owned. These hangars consist of the 68,800-square foot clear-span hangar (main hangar), one small 1,900-square foot box hangar, and one 14-unit Thangar building, of which all are leased to maximum capacity as reported by Airport Staff. The Airport leases half of the main hangar to Ocean Air Aviation, while the other half is leased to individual based aircraft tenants. Airport records also indicate there is a total of four aircraft owners on the T-hangar wait list. Analysis conducted to determine the amount of total hangar space and type of hangar facilities required for the planning period assumed that additional hangar demand during the planning period is likely to come from three sources: pent-up demand among owners, the FBOs desire for additional leasable space, and demand generated by owners moving up to larger aircraft.

Based aircraft that cannot be accommodated in T-hangars or box/corporate hangars are stored in the main hangar. Demand for large conventional hangars would typically be driven by an aviationoriented business or a corporate flight department. The 2002 Airport Master Plan discussed the potential replacement of the 68,800-square foot main hangar. The main hangar was built in 1941, constructed of wood, and is in close proximity to the fuel farm. Currently, there is no sprinkler system or other fire suppression system installed inside the main hangar, which does not meet the Oregon Fire Code for aircraft-related occupancies. The main hangar's doors do not close and approximately 38 percent of the total square footage is comprised of circulation and unusable space, making a significant portion of the hangar nonfunctional for its intended purpose to store aircraft.

As mentioned previously, half of the main hangar is leased to Ocean Air Aviation. Interviews with Ocean Air indicate that overnight storage during the peak summer months typically reaches maximum capacity inside the main hangar. Peak demand was included for the analysis. **Chapter 3, Aviation Demand Forecasts**, concluded that 11 new based aircraft are anticipated by 2030. Further, using planning standards established and accepted by the Airport in the 2002 Master Plan Update and 2004 Airport Layout Plan Revision, required square footage allocated by aircraft type was 1,100 square feet for single engine aircraft, and 2,000 square feet for multi-engine, turboprop, helicopter, and business jet aircraft. Two scenarios were developed to analyze the demand for additional hangars, including large conventional hangars, during the 20-year planning period. In Table 4-19, Scenario One shows the existing and forecast demand for hangar storage, assuming the main hangar will not be removed. As the main hangar has exceeded its functional life, Scenario Two, shown in Table 4-20, assumes the main hangar will be removed in the short-term and not replaced, to demonstrate the demand for hangar storage.

Medium box and corporate hangars are typically used for small/medium business jets, large piston twins, helicopters, and turboprop aircraft. The analysis shown in Table 4-19 and Table 4-20 that existing box/corporate hangar space can accommodate demand throughout the planning period. Demand for small conventional hangars is largely price-driven and based on the aircraft owners' plans for the hangar in addition to aircraft storage.

T-hangars are a popular choice for small aircraft owners at many airports. T-hangars house single aircraft in a staggered, back-to-back configuration that maximizes the number of aircraft that can be hangared in a given area, while still giving each lessor private storage area and each aircraft



direct access to the ramp. Both forecast facility requirements in Scenario One and Scenario Two indicate the market could support an additional multi-unit T-hangar that would accommodate current and future demand.

	Actual		Projected		
Scenario One	2010	2015	2020	2025	2030
Total Based Aircraft	51	54	58	62	62
T-Hangar Square Footage (SF)					
Required SF	19,800	19,800	19,800	19,800	19,800
Existing SF Used	15,400	15,400	15,400	15,400	15,400
Additional SF Needed Surplus / (Deficit)	(4,400)	(4,400)	(4,400)	(4,400)	(4,400)
Box/Corporate Hangar SF					
Required SF	32,200	32,200	32,200	32,200	32,200
Existing SF Used	42,500	42,500	42,500	42,500	42,500
Additional SF Needed Surplus / (Deficit)	10,300	10,300	10,300	10,300	10,300
Main Hangar SF					
Total Hangar SF	68,800	68,800	68,800	68,800	68,800
Circulation/Unusable Storage SF	26,000	26,000	26,000	26,000	26,000
Leased/Reserved for Peak Overnight Storage SF	21,400	21,400	21,400	21,400	21,400
Required SF	22,000	26,200	31,500	36,800	36,800
Existing SF Used	21,400	21,400	21,400	21,400	21,400
Additional SF Needed Surplus / (Deficit)	(600)	(4,800)	(10,100)	(15,400)	(15,400)
Reserved Maintenance Storage SF					
Required SF (15% of Total Required SF)	11,243	11,881	12,686	13,492	13,492
Existing SF Used	10,300	10,300	10,300	10,300	10,300
Additional SF Needed Surplus / (Deficit)	(943)	(1,581)	(2,386)	(3,192)	(3,192)

# Table 4-19 HANGAR STORAGE REQUIREMENTS – SCENARIO ONE

Source: Reynolds, Smith and Hills, Inc., 2012.



Table 4-20
HANGAR STORAGE REQUIREMENTS – SCENARIO TWO

	Actual	Projected				
Scenario One	2010	2015	2020	2025	2030	
Total Based Aircraft	51	54	58	62	62	
T-Hangar Square Footage (SF)						
Required SF	19,800	19,800	19,800	19,800	19,800	
Existing SF Used	15,400	15,400	15,400	15,400	15,400	
Additional SF Needed Surplus / (Deficit)	(4,400)	(4,400)	(4,400)	(4,400)	(4,400)	
Box/Corporate Hangar SF						
Required SF	32,200	32,200	32,200	32,200	32,200	
Existing SF Used	42,500	42,500	42,500	42,500	42,500	
Additional SF Needed Surplus / (Deficit)	10,300	10,300	10,300	10,300	10,300	
Main Hangar SF						
Total Hangar SF	68,800	0	0	0	0	
Circulation/Unusable Storage SF	26,000	0	0	0	0	
Leased/Reserved for Peak Overnight Storage SF	21,400	(21,400)	(21,400)	(21,400)	(21,400)	
Required SF	22,000	26,200	31,500	36,800	36,800	
Existing SF Used	21,400	0	0	0	0	
Additional SF Needed Surplus / (Deficit)	(600)	(26,200)	(31,500)	(36,800)	(36,800)	
Reserved Maintenance Storage SF						
Required SF (15% of Total Required SF)	11,243	11,881	12,686	13,492	13,492	
Existing SF Used	10,300	10,300	10,300	10,300	10,300	
Additional SF Needed Surplus / (Deficit)	(943)	(1,581)	(2,386)	(3,192)	(3,192)	

Source: Reynolds, Smith and Hills, Inc., 2012.

In both scenarios, new aircraft cannot be accommodated in hangars, and T-hanger demand for additional space remains constant from the base year throughout the 20 years. In Scenario One, hangar storage in the main hangar is currently deficient, and the demand for aircraft storage in the main hangar increases to 15,400 square feet deficit by the end of the planning period. Although the surplus in box/corporate hangars is used for maintenance storage, an estimated 15 percent of total hangar was used as the standard requirement for future years. As shown in both scenarios, the current shortage in maintenance storage is expected to increase throughout the 20-year planning period.

In Scenario Two, without the main hangar, the current need grows to 36,800 square feet required to accommodate existing and new aircraft in hangars. Without the main hangar, multi-engine aircraft overnighting during the peak season may also trigger the need for additional box hangars. Further, 21,400 square feet used for peak overnight storage for transient aircraft would also need to be accommodated. Remaining box/corporate hangar space is currently utilized for maintenance storage; however, the existing capacity constraints indicate a deficit in maintenance storage space throughout the planning period.

With the existing demand for aircraft hangar storage, the location and size of additional hangars will be analyzed in the Alternatives Analysis chapter. The Alternatives Analysis will also further examine the size and location of a potential replacement hangar(s) for the main hangar to accommodate future demand.



## 4.9.5 General Aviation Aprons

The general aviation aprons at the Airport provide areas for aircraft parking, fueling, and tie-downs, as well as aircraft movement to and from parking and storage hangars. Demand for general aviation apron space is primarily driven by itinerant aircraft not desiring hangar storage.

FAA Advisory Circular 150/5300-13, Appendix 5, provides a methodology by which apron requirements can be determined for general aviation facilities. Analysis conducted to determine parking requirements for itinerant aircraft calculated daily parking demand by focusing on the peak month of operations. FAA planning criteria recommends the amount of itinerant aircraft parking positions needed at one time be approximately 50 percent of the peak day itinerant operating aircraft. Currently, aircraft dwell time is a significant contributor to the Airport's apron congestion. Increased dwell time occurs when aircraft remain parked on the ramp longer than 24 hours, instead of departing shortly after arriving passengers have been deplaned. At OTH, transient aircraft frequently stay parked more than 24 hours, and sometimes up to a week, occupying apron space that would otherwise be available for arriving traffic. Thus, a higher percentage than 50 percent of the peak day itinerant operating aircraft was used to determine required apron parking positions.

In addition, this anlaysis also assumed that the peak month will increase in proportion to the year over year percent change in operations. Considering both the local and itinerant aircraft fleet mix, as previously adopted by the Airport in the 2004 Airport Layout Plan Revision, a planning standard of approximately 1,700 square yards of apron should be provided for each itinerant aircraft position (including helicopters), and approximately 580 square yards should be provided for each single engine aircraft tie-down position. Airport records indicate there are approximately 12 anchored tie-downs on the main ramp, with one reserved as the FedEx hard stand for cargo operations. On the remaining 22,070 square yards available for itinerant aircraft parking, approximately 13 jets could be parked on the main apron. Table 4-21 provides the aircraft parking apron required to accommodate forecasted demand during the 20-year planning period.



Apron Requirements	2010	2015	2020	2025	2030
Total Main Apron (SY)	30,150	30,150	30,150	30,150	30,150
Total South GA / Itinerant Overflow Apron (SY)	7,939	7,939	7,939	7,939	7,939
Local Aircraft Parking					
Local Tie-Down Requirement (SY)	6,380	6,380	6,380	6,380	6,380
FedEx Hard Stand Requirement (SY)	1,700	1,700	1,700	1,700	1,700
Total Local Apron Requirement (SY)	8,080	8,080	8,080	8,080	8,080
Available Tie-Down Positions	12	12	12	12	12
Local Tie-Down Positions Requirement	9	9	9	9	10
Total Local Tie-Down Positions Surplus / (Deficit)	3	3	3	3	2
Itinerant Aircraft Parking					
Itinerant Peak Day Operating Aircraft	30	33	35	37	40
Itinerant Apron Available on Main Apron (SY)	22,070	22,070	22,070	22,070	22,070
Itinerant Apron Requirement (SY)	33,691	36,761	38,376	41,642	44,616
Total Itinerant Apron Requirement (SY) Surplus / (Deficit) <sup>(1)</sup>	(3,682)	(6,752)	(8,367)	(11,633)	(14,607)
Available Itinerant Aircraft Parking Positions <sup>(2)</sup>	18	18	18	18	18
Itinerant Aircraft Parking Positions Requirement	23	25	26	28	30
Total Itinerant Aircraft Positions Surplus / (Deficit)	(5)	(7)	(8)	(10)	(12)

Table 4-21 AIRCRAFT PARKING APRON REQUIREMENTS

Source: RS&H Analysis, 2012.

<sup>(1)</sup> Includes South GA / Itinerant Overflow Apron

<sup>(2)</sup> Includes 13 jet positions on main apron, and five small aircraft parking positions located on the South GA/Itinerant Overflow Aprons. The South GA/Itinerant Overflow Apron has three anchored and two non-anchored tie-down positions.

Cumulatively, the total local and itinerant apron need is over 52,000 square yards over the 20-year planning period. The local apron requirement throughout the planning period is approximately 8,080 square yards to accommodate the existing 11 anchored tie-down positions for based aircraft, and the FedEx hard stand. However, remaining space on the main apron available for itinerant use combined with the South General Aviation apron area available for overflow parking, currently cannot accmodate demand for itinerant aircraft parking. The existing apron shortage of 3,682 square yards will increase to 14,607 square yards by 2030, equating to at least 12 aircraft parking positions.

During consultation with the Master Plan Advisory Committee (MPAC), it was indicated that on an average annual basis, there is generally sufficient parking apron available to meet existing demand; however, peak demand can occur more frequently even outside of the peak season. Further, the MPAC reports that tie-down space is especially limited for smaller general aviation aircraft during the peak season, as several business jets are tightly parked over existing tie-down positions. The MPAC also expressed that there can be nearly 30 jets on the apron, and at times are required to turn away overnight transient users when the main apron is at full capacity. Development alternatives to accommodate existing and forecasted peak demand for apron space will be examined in the next chapter.



## 4.9.6 <u>Airport Fuel Storage</u>

Fuel storage requirements at the Airport depend on the level of aircraft traffic, fleet mix, and fuel delivery schedules. Changes in aircraft fleet mix, for example, air carrier turboprops being replaced by jets, or piston twins being replaced by turboprops, will likely increase demand for Jet-A. Table 4-22 outlines fuel storage requirements for the 20-year planning period.

The current total storage capacity for 100LL avgas is 9,500 gallons and 22,000 gallons for Jet-A fuel. Based upon average bulk fuel delivery estimates reported by the FBOs during the peak months, the five-day peak supply requirement for 100LL is approximately 981 gallons, and is forecast to decrease to 311 gallons by 2030. The five-day peak supply requirement for Jet-A is currently approximately 22,350 gallons, and is forecast to increase to 76,038 gallons by 2030. Increases (or decreases) in airline service will also affect future Jet-A storage needs. Appropriate storage for 100LL avgas appears adequate throughout the forecast period, while a deficit in Jet-A storage capacity increases to a 54,038 gallon need by 2030. This does not include any fuel storage capacities contained within fuel trucks. The growing need for Jet-A fuel storage capacity is a result of increasing peak month general aviation operations and commercial jet operations, as turboprops are replaced with regional jets during the planning period. As shown below, existing Jet-A fuel storage capacity is not adequate to accommodate peak season demand.

	2010	2015	2020	2025	2030
Peak Month Average Day (PMAD) Operations	58	56	58	61	63
100 LL AvGas					
PMAD Fuel Requirement	196	161	129	131	62
5 Day Fuel Need (gallons)	981	806	646	653	311
Available Storage (gallons)	9,500	9,500	9,500	9,500	9,500
Additional Storage Needed Surplus / (Deficit)	8,519	8,694	8,854	8,847	9,189
Jet-A					
PMAD Fuel Requirement	4,470	6,354	9,187	12,032	15,208
5 Day Fuel Need (gallons)	22,350	31,769	45,934	60,159	76,038
Available Storage (gallons)	22,000	22,000	22,000	22,000	22,000
Additional Storage Needed Surplus / (Deficit)	(350)	(9,769)	(23,934)	(38,159)	(54,038

#### Table 4-22 FUEL FACILITY REQUIREMENTS

Source: Reynolds, Smith and Hills, Inc., 2012.

As required by the Environmental Protection Agency (EPA), Title 40 of the Code of Federal Regulations (CFR) Part 112, a Spill Prevention Control, and Countermeasure (SPCC) Plan is required for aboveground oil storage capacity exceeding 1,320 gallons.

## 4.9.7 <u>Aircraft Rescue and Fire Fighting Equipment and Facility</u>

Airports that serve air carrier flights are required to provide Aircraft Rescue and Firefighting (ARFF) facilities and equipment. ARFF equipment requirements for FAR Part 139 airports are determined by an index ranking based on aircraft size, number of emergency vehicles, and number of scheduled daily aircraft departures. As published by the FAA, Southwest Oregon Regional Airport



is FAR Part 139 Class I, with an ARFF Index A, and on request by contacting the Airport Manager, the Airport can meet Index B requirements.

The existing 3,786 square foot ARFF building was built in 1960, and is in poor condition. The ARFF building is centrally located immediately south of the main hangar, on the southwest side of the main apron. Currently, the ARFF building houses two fire rescue vehicles, and essential equipment. As the building has exceeded its useful life and service condition, the ARFF building should be replaced with one that has the ability to accommodate two trucks, fire station boats, and ARFF personnel and equipment based on design standards specified in FAA AC 150/5210-15A, *Aircraft Rescue and Firefighting Station Building Design.* 

The primary ARFF vehicle is a 2002 all-wheel drive KME truck, with a capacity of 1,500 gallons of water and 250 gallons of foam. This vehicle meets FAA Index B requirements, which include the capability of holding 1,500 gallons of water and 200 gallons of foam. In addition, the older 1984 Ameritek ARFF truck will require replacement during the planning period based upon FAA AC 160/5220-10E, *Guide Specification for Aircraft Rescue and Fire Fighting Vehicles.* 

## 4.9.8 <u>Airport Maintenance Equipment Facility</u>

The demand for airport maintenance facilities is directly related to the amount of pavement, lighting equipment, terminal building size, and overall grounds maintenance airport staff is required to maintain. Since the airport maintenance building was constructed in 1941, the condition of the building is deteriorating and may need to be replaced at some point during the planning period. It can also be assumed that as the airfield and/or facilities are enlarged, the maintenance facilities may also need to be expanded and perhaps relocated. Although no large-scale infrastructure additions are anticipated, the maintenance facility at Southwest Oregon Regional Airport has exceeded its useful life and is considered inadequate for the 20-year planning period.

### 4.9.9 Deicing and Snow Removal

Aircraft deicing facilities are recommended at airports where icing conditions are expected. With the area's relatively mild winters, deicing activity at Southwest Oregon Regional Airport is rare, although the Airport has deicing capabilities. When deicing is necessary, commercial aircraft are deiced by United/SkyWest Airlines.

The Airport's snow removal equipment includes a 2009 TYM 903 4-wheel drive tractor with a halfyard bucket, and a 1993 Ford 660 2x4 tractor with seven-foot roll broom attachment, and is considered adequate to accommodate future snow removal activities.

Runoff from the deicing process has the potential to cause environmental degradation in nearby waterways. Consideration should be given to creating a mitigation plan to ensure minimal deicing fluid reaches waterways.



## 4.10 ENVIRONMENTAL CONDITIONS

Existing environmental categories identified as a potential concern to the Airport's development include coastal resources, fish, wildlife, and plants, and wetlands. Due to the Airport's location adjacent to Coos Bay, there are sensitive environmental issues to consider for any potential subsequent NEPA process. A few examples of such scenarios are as follows:

- Future development that may conflict with policies and planning goals of the Oregon Coastal Zone Management Program, which enforces the Coastal Zone Management Act (CZMA). Agency coordination with the Oregon Department of Land Conservation and Development would be recommended for any projects affecting the Coos Bay.
- Land expansion into Coos Bay may affect the habitats for groundfish, coastal pelagic species, and Pacific salmon, which are protected under the federal Fishery Management Plan (FMP) in Oregon. Coordination with the U.S. Fish and Wildlife Service (USFWS) would be necessary to determine the potential impacts to these habitats.
- Construction on- or off-airport property may disturb "jurisdictional" or "non-jurisdictional" wetlands defined by the U.S Army Corps of Engineers (USACE) or the Oregon Department of State Lands (DSL). A wetland delineation report (WDR) conducted by a wetland specialist would define the extent and impact of such resources and possible mitigation measures if needed. Agency coordination with USACE and DSL is recommended prior to any design or construction efforts given the presence of wetlands surrounding the Airport.

Most of the projects listed in Table 4-23 would likely require categorical exclusion (CatEx) documentation under NEPA. However, given the sensitive nature of the environmental impact categories discussed above, potential development may result in an Environmental Assessment (EA). The Airport should coordinate with the local FAA ADO on the appropriate course of action during the initial design phase for the project. An analysis of each potential development project and its effect on each environmental impact category will be discussed in Chapter 5.



## 4.11 SUMMARY OF MAJOR AIRPORT FACILITY NEEDS

The airport facilities requirements needed to adequately accommodate the forecast activity, address FAA design standards, and meet the strategic goals for the Airport have been addressed within this chapter. Certain facility requirements identified in this chapter will require further analysis to determine the optimum layout and potential, which will be evaluated in **Chapter 5**, **Alternatives Analysis**. The summary of major airport facility needs are listed below in Table 4-23.

Item	Identified Need		
Runway Safety Area (RSA)	Runway 22 RSA does not meet FAA design standards for C-III		
Object Free Area (OFA)	Runway 22 OFA does not meet FAA design standards for C-III		
Object Free Area (OFA)	Runway 31 OFA does not meet FAA design standards for C-III		
Object Free Area (OFA)	A portion of the East Perimeter Road is located inside the Runway 31 OFA		
Runway Protection Zones (RPZs)	Acquire in fee simple or easement property within the Runway 04, 22, 13, 31 RPZs not controlled by the Airport		
Taxiway B	Increase Taxiway B width by eight feet to meet ADG-III 50-foot width design standard		
Taxiway C	Runway 13 and Taxiway C intersection FAA designated "Hotspot"		
Taxiway A1, and A2	Taxiways do not meet Engineering Brief 75 recommended guidance		
Taxiway Shoulders	Add 20-foot wide stabilized or paved shoulders to Taxiways A, B, C, D, F, and G to meet ADG-III design standard		
Pavement Strength and Condition	Implement Pavement Maintenance Plan for all airfield pavement		
Visual Aids	Install REILs to Runway 22		
Visual Aids	Upgrade Runway 04 and Runway 13 VASIs to PAPIs		
Visual Aids	Upgrade Taxiway K edge reflectors to MITLs		
T-Hangars	An additional 4,400 SF are needed		
Conventional Hangars	An additional 15,400 SF are needed		
Aircraft Apron	An additional 2,911 SY are needed		
ARFF Building	Replace ARFF building due useful life and insufficient space		
ARFF Equipment	Replace the 1984 Ameritek ARFF truck		
Airport Maintance Building	Replace building due to useful life and insufficient space		
Vehicle Access	Improve airport entrance vehicle wayfinding and signage		

## Table 4-23 SUMMARY OF MAJOR AIRPORT FACILITY NEEDS

Source: RS&H, 2012.

